#### 100 Yr. H.W. Elev. 518.4 Traffic Barrier Terminal, Type 5A Steel Railing, Type S1 20 Yr, H.W. Elev. 516.5 See Std. BLR27 (Typ.) Name Plate See sheets 16 & 20 for details. Porous Granular Embankment (Typ.) Elev. 518.0 (Typ.) Elev. 508.0± Elev. 503.0± Elev. 505.0± Channel Excavation Riprap Special Æ Elev. 502.2

ELEVATION

# 85'-278" Bk. - Bk. Abuts. # Care dans dans dans dans 81'-7'4" Fc. - Fc. Abuts. Flow a a Bk. North Abut. Bk. South Abut. Sta. 34+67.94 Sta. 35+10.56 Sta. 35+53.18 Elev. 522.70 Elev. 522.70 <del>À À À À</del> Limits of Existing 25'-0" Transition Name Plate Concrete Bridge Deck See Std. for details. (Typ.) PLAN

# DESIGN STRESSES

FIELD UNITS (SUPERSTRUCTURE)

f'c = 3,500 psi fy = 60,000 psi (Reinf.) fy = 50,000 p.s.i. (Struct.)

#### SEISMIC DATA

DESIGNED - A.S.L CHECKED - S.W.M. DRAWN - D.A.B. CHECKED - S.W.M. Seismic Performance Zone (SPZ) = 1 Design Spectral Acceleration at 1.0 sec. ( $S_{D1}$ ) = 0.053g Design Spectral Acceleration at 0.2 sec. ( $S_{D5}$ ) = 0.129g

#### LOADING HL-93 (SLAB)

Design Specifications: 2007 AASHTO LRFD with all applicable interims.
50#/Sq. Ft. included in dead load for future wearing surface.

## WATERWAY INFORMATION

Drainage Area = 15.4 Sq. Mi. Existing Low Grade Elev. 522.0 © Sta. 35+10.56 Proposed Low Grade Elev. 522.7 © Sta. 35+10.56									
Flood	Freq. Q		Opening Sq. Ft.		Natural	Head	- Ft.	Headwater El.	
F1000	Yr.	C.F.S.	Exist.	Prop.	H.W.E.	Exist.	Prop.	Exist.	Prop.
	10	2410	720	720	515.11	0.27	0.27	515.38	515.38
Design	20	3020	820	820	516.51	0.38	0.38	516.89	516.89
Base	100	4500	970	970	518.37	0.60	0.60	518.97	518.97
Max. Calc.	500	6080	1030	1030	519.71	0.41	0.41	520.12	520.12
10 Year Velocity th	rough Exis	tina Bridae	= 3.3 fps	10	Year Velocit	ty through	Proposed .	Bridge = 3	3 fps

# GENERAL NOTES

Plan dimensions and details relative to existing structure have been taken from field survey and are subject to nominal construction variations. It shall be the Contractor's responsibility to verify such dimensions and details in the field and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in the scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.

The existing structural steel coating contains lead. The Contractor shall take appropriate precautions to deal with the presence of lead on this project. The Contractor shall excavate behind the abutments to the elevations

shown prior to the removal of the existing superstructure. The excavation shall be backfilled with Porous Granular Backfill after superstructure construction has been completed. The excavation required behind the abutments shall be included with Porous Granular Backfill.

Layout of the slope protection system may be varied to suit ground conditions.

in the field as directed by the Engineer.

The SSPC-QP1 Painting Contractor Certification will be

required for this bridge.

Fequired for this bridge.

Fasteners shall be high strength bolts. (AASHTO M164, Type 3 in unpainted areas and mechanically galvanized AASHTO M 164, Type 1 or 2 in painted areas). Bolts 3/0, open holes 15,6 0, unless otherwise noted.

All structural steel and exposed surfaces of bearings within a distance of

6 ft. each way from the deck joints shall be painted as specified in the Special Provision for "Surface Preparation and Painting Requirements for

Weathering Steel".

Reinforcement bars shall conform to the requirements of ASTM A 706

Reinforcement bars shall conform to the requirements of ASTM A 706 Grade 60. See Special Provisions.

All construction joints shall be bonded, except as noted.

Protective Coat shall be applied to the bridge surface and fascia.

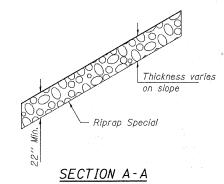
Field welding of construction accessories to beams will not be permitted.

All proposed construction activity shall be in accordance with Nationwide Permit number 14 of the Department of the Army authorized under Section 404 of the Clean Water Act. The IEPA has issued Section 401 Water Quality Certification for this activity. See Special Provisions for conditions.

All structural steel shall be AASHTO M 270 Grade 50W. All structural steel shall be cleaned as specified in the Special Provision for "Surface Preparation and Paintina Requirements for Weathering Steel".

and Painting Requirements for Weathering Steel".

Calculated weight of structural steel = 40,309 pounds.



R. 2 W., 4TH. P.M.

LOCATION SKETCH

LOGAN CREEK

BUILT 201\_ BY

BROWN COUNTY

SEC. 05-00065-00-BR C.H. 3A / FAS 1583 STR. NO. 005-3006

LOADING HL-93

NAME PLATE See Std. 515001

Proposed Bridge

### TOTAL BILL OF MATERIAL

		_		
ITEM	UNIT	SUPER	SUB	TOTAL
Channel Excavation	Cu. Yd.			100
Porous Granular Backfill	Ton		70	70
Riprap Special	Ton			100
Removal of Existing Superstructure	Each	1		1
Concrete Structures	Cu. Yd.		3.0	3.0
Concrete Superstructure	Cu. Yd.	67.8		67.8
Protective Coat	Sq. Yd.	272		272
Furnishing and Erecting Structural Steel	L. Sum		1	1
Stud Shear Connectors	Each	1,860		1,860
Reinforcement Bars, Epoxy Coated	Pound	13,020	230	13,250
Steel Railing, Type S1	Foot	171		171
Name Plates	Each	1		1
Preformed Joint Strip Seal	Foot	27		27
Elastomeric Bearing Assembly, Type 1	Each	10		10
Anchor Bolts, 1"	Each	40		40

I certify that to the best of my knowledge, information and belief, this bridge design is structurally adequate for the design loading shown on the plans. The design is an economical one for the style of structure and complies with requirements of the current "AASHTO LRFD Specifications."

Muhael B. Berry 4/09/10 ILLINOIS STRUCTURAL NO. 081-4622



Expires 11-30-2010

## GENERAL PLAN AND ELEVATION STRUCTURE NO. 005-3006

HAMPTON, LENZINI & RENWICK, INC. 3085 STEVENSON DRIVE, SUITE 201 SPRINGFIELD, ILLINOIS 62703

(217) 546-3400

C.H.	C.H.   SECTION			COUNTY		SHEETS	NO.
3A	05-00065-00-BR			BROWN		24	12
				CONTRA	4CT	NO. 9:	509
FED. R	OAD DIST. NO	. ILLINOIS	FED. A	ID PROJECT	ARA	1583(103)	